

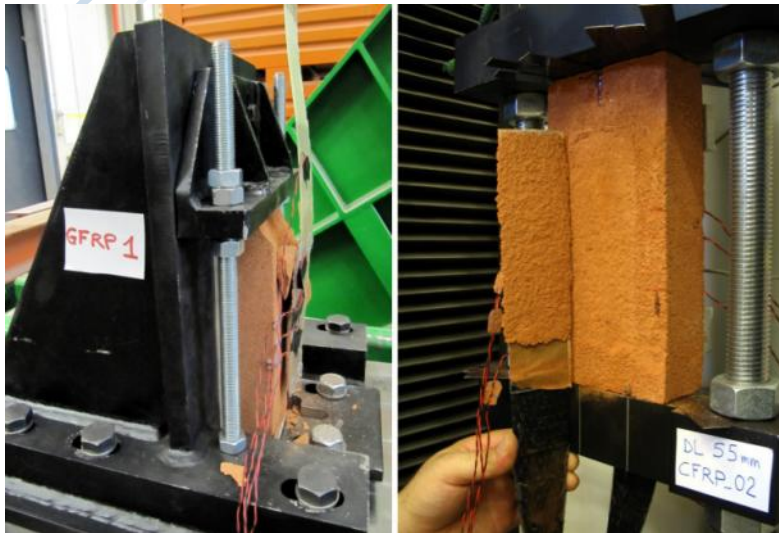


# EXPERIMENTAL INVESTIGATION ON BOND OF FRP/SRP APPLIED TO MASONRY PRISMS

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# Background



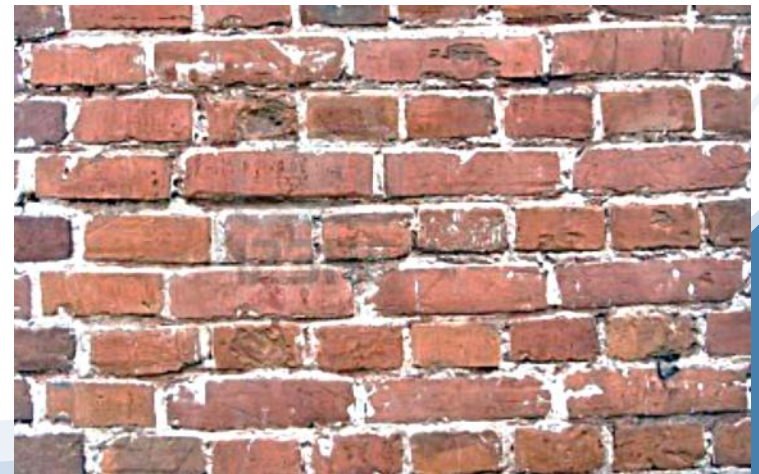
Investigations of bond behaviour on masonry elements (clay bricks, natural stones,...) are increasing.

Typical effective lengths for solid clay bricks may be about  $80 \div 100\text{mm}$  (glass...),  $120 \div 150\text{mm}$  (carbon...).

Height of solid clay bricks is commonly about  $40 \div 60\text{mm}$ .

Joints of historic masonry are generally made of poor lime mortars.

The role of mortar joints still needs to be deepened.

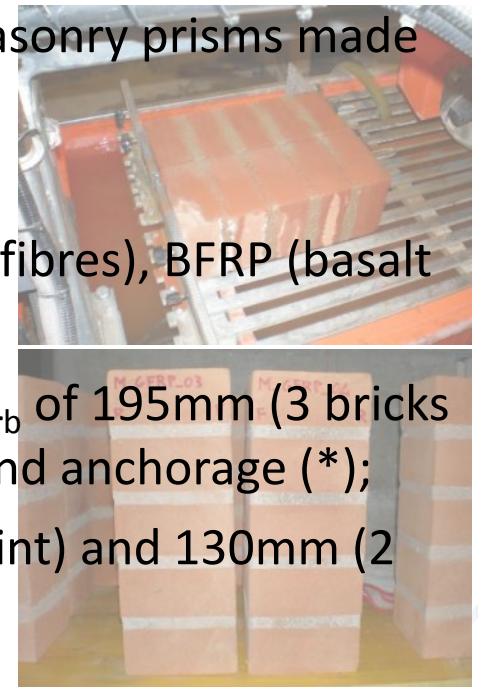




# Experimental activity

Overall thirty Single-lap (SL) Shear Tests on five-course masonry prisms made of solid facing clay bricks and weak lime mortar:

- ❑ **one type** of masonry substrate;
- ❑ **four types** of epoxy-based reinforcement, GFRP (glass fibres), BFRP (basalt fibres), CFRP (carbon fibres) and SRP (steel fibres);
- ❑ **two types** of specimens, one having a bonded length  $L_b$  of 195mm (3 bricks and 3 joints) and the second having a reinforcement end anchorage (\*);
- ❑ **additional shorter  $L_b$**  for GFRP, 65mm (1 brick and 1 joint) and 130mm (2 bricks and 2 joints).



## TEST MATRIX

REINFORCEMENT	$L_b = 65$ mm	$L_b = 130$ mm	$L_b = 195$ mm	end-anchored
GFRP	3	3	3	3
BFRP			3	3
CFRP			3	3
SRP			3	3



# Materials characterization



## Mortar Tassullo T30V:

- ☐  $f_{c,T30V} = 2.6 \text{ N/mm}^2$
- ☐  $f_{sp,T30V} = 0.35 \text{ N/mm}^2$
- ☐  $E_{T30V} = 5490 \text{ N/mm}^2$

## Bricks San Marco Rosso Vivo:

- ☐  $f_{c,b} = 19.8 \text{ N/mm}^2$
- ☐  $f_{sp,b} = 2.5 \text{ N/mm}^2$
- ☐  $E_b = 5760 \text{ N/mm}^2$



## Masonry assemblage:

- ☐  $f_{c,m} = 8.2 \text{ N/mm}^2$
- ☐  $f_{sp,m} = 1.1 \text{ N/mm}^2$
- ☐  $E_m = 2060 \text{ N/mm}^2$



## Impregnated composites

### (Fidia s.r.l.):

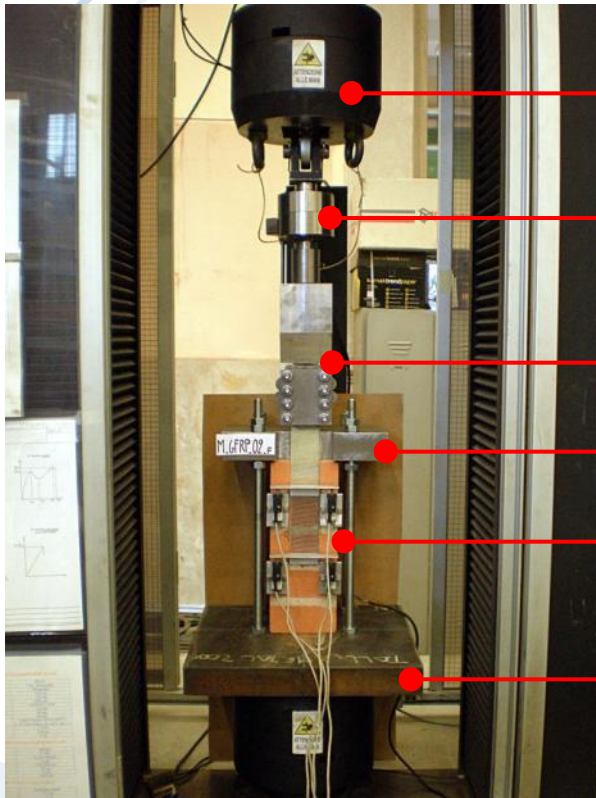
- ☐  $E_{GFRP} = 80 \cdot 10^3 \text{ N/mm}^2$
- ☐  $E_{BFRP} = 87 \cdot 10^3 \text{ N/mm}^2$
- ☐  $E_{CFRP} = 241 \cdot 10^3 \text{ N/mm}^2$
- ☐  $E_{SRP} = 200 \cdot 10^3 \text{ N/mm}^2$







# Test setup and instrumentation



movable transverse beam

load cell

clamping system

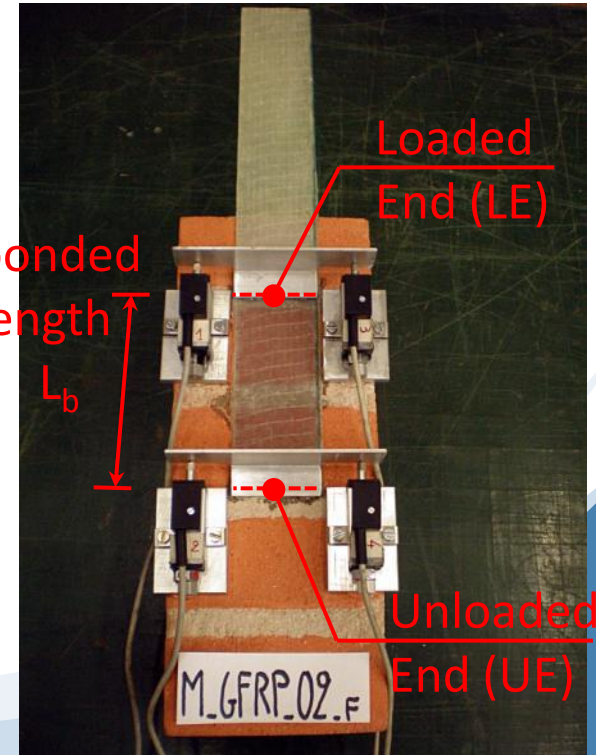
upper steel plate

sample

lower plate fixed to the machine's head

Single-lap setup  
disp. rate of 0.3 mm/min  
acquisition rate of 10 Hz

two potentiometers at LE  
and other two at UE





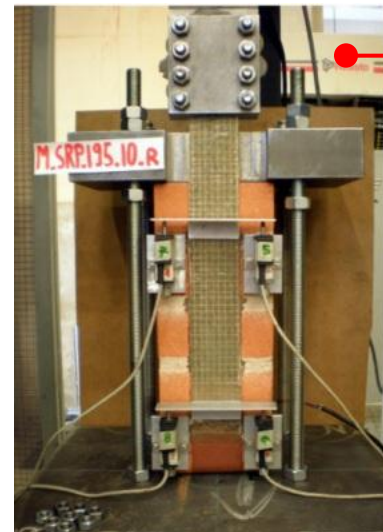
# Specimens



GFRP, with  $L_b = 65\text{mm}$  → 1 brick + 1 mortar joint



GFRP, with  $L_b = 130\text{mm}$  → 2 bricks + 2 joints



SRP,  $L_b = 195\text{mm}$   
↓  
3 bricks + 3 joints



end-anchored CFRP → 3 bricks + 3 joints + anchorage



# Typical failures



GFRP, with  $L_b = 65\text{mm}$



GFRP, with  $L_b = 130\text{mm}$



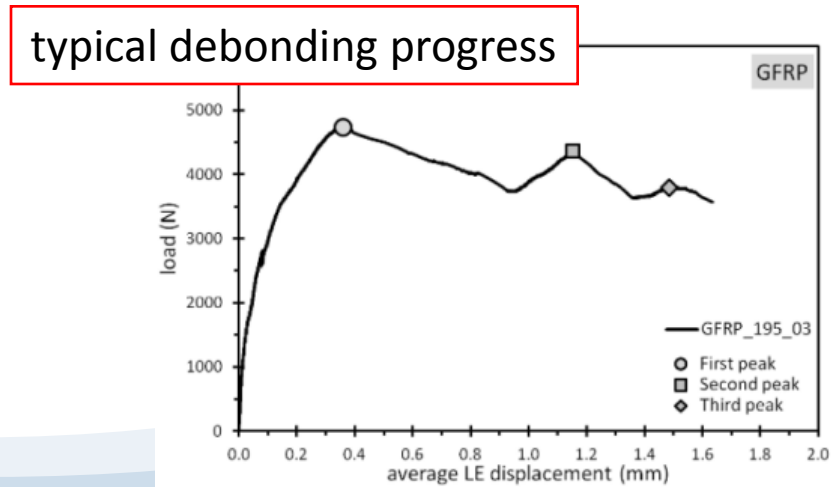
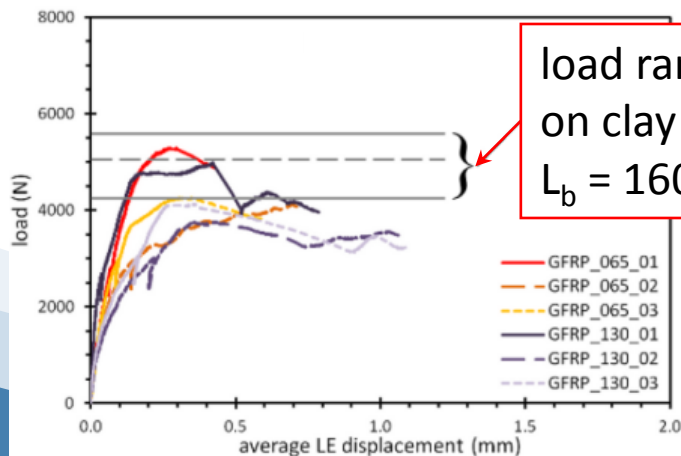
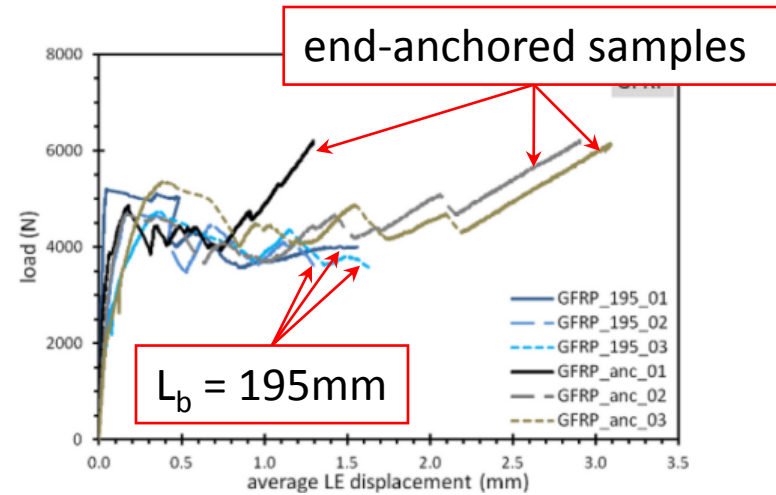
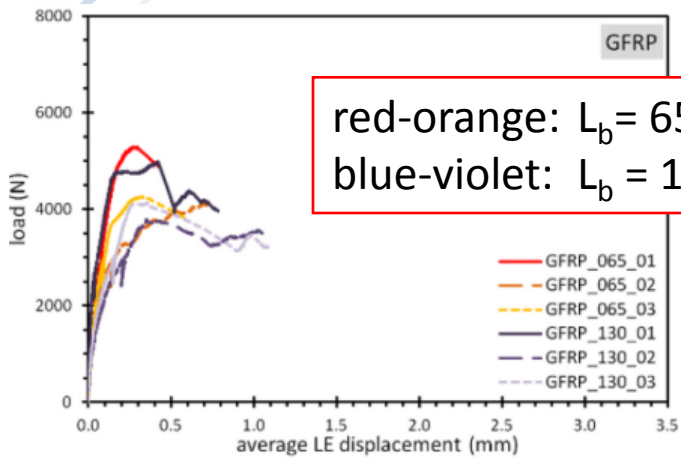
end-anchored CFRP



end-anchored SRP



# Load results for GFRP (various $L_b$ )

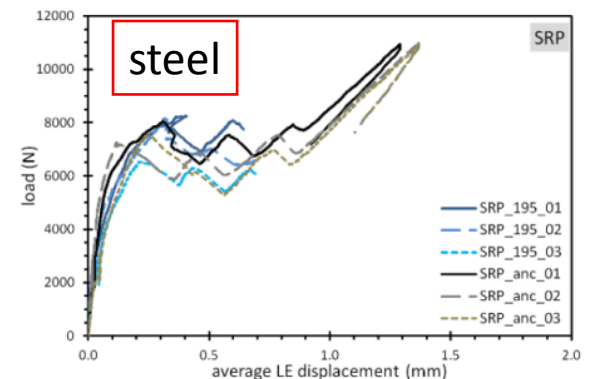
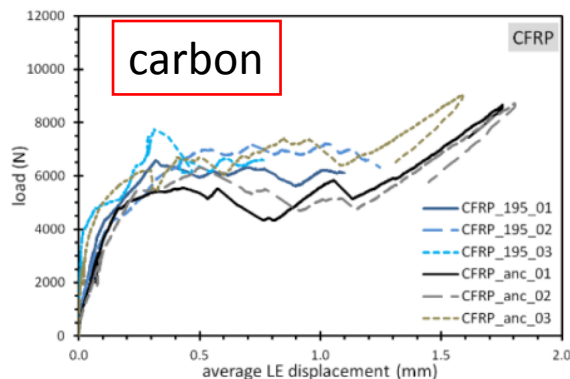
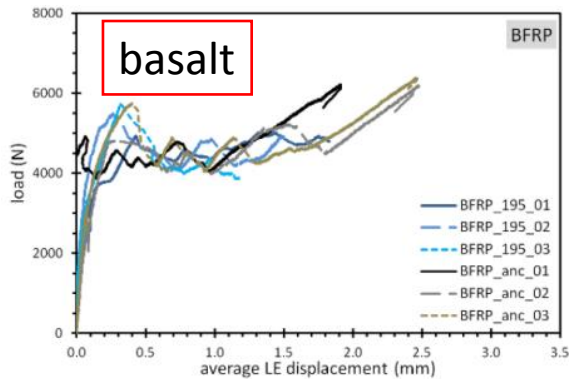




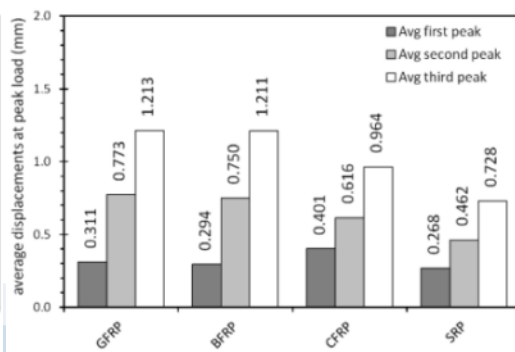
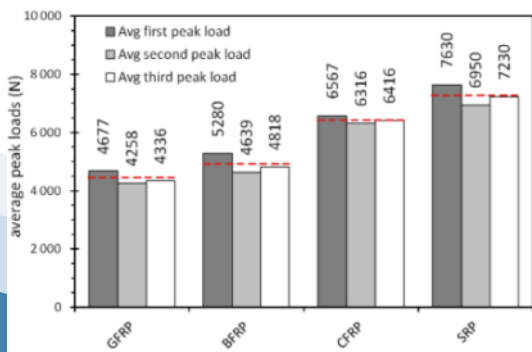


# Results for BFRP, CFRP and SRP

## load – LE displacement curves



## peak loads and related LE displacements



### Peak loads:

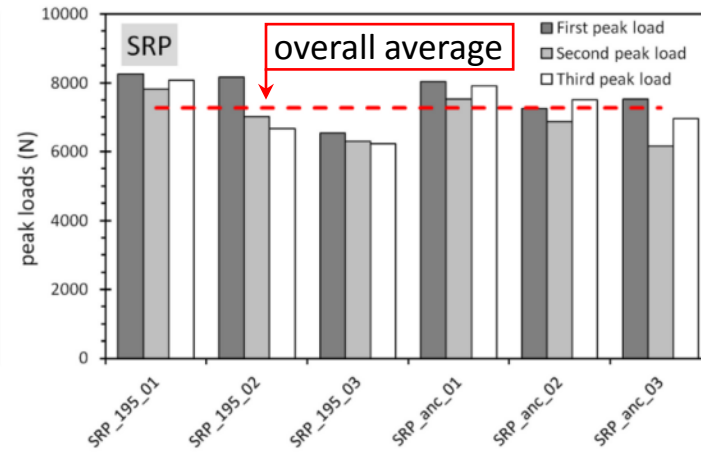
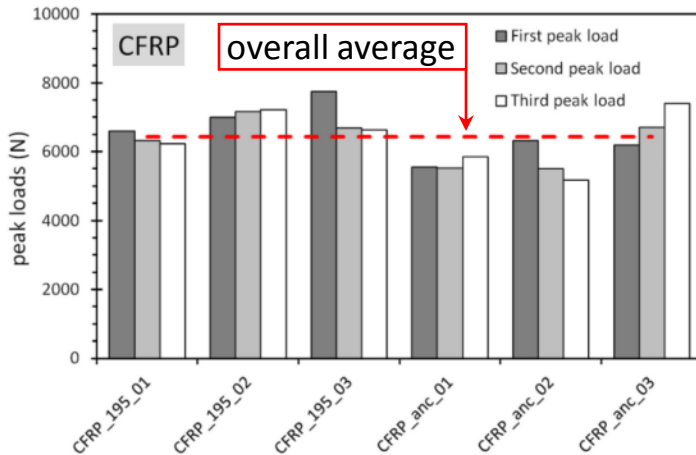
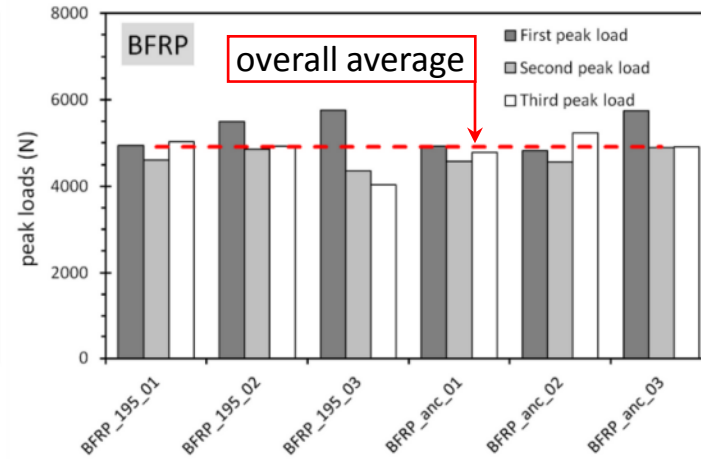
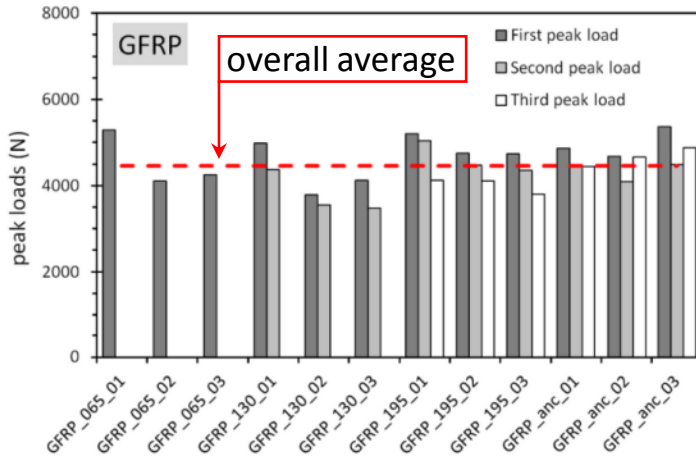
- slight prevalence of the 1<sup>st</sup> one.

### LE displacement:

- No significant difference for the 1<sup>st</sup> peak;
- consistent with different stiffness for subsequent ones



# Recorded peak loads





# Conclusions

- ❑ For *that combination* of clay bricks and weaker lime mortar (similar elastic modulus but rather different strength), the presence of mortar joints has a strong influence on the bond behaviour: it appears that higher bonded lengths do not provide higher strength, as if joints split the bonded area into segments one-brick long.
- ❑ strengths recorded with  $L_b$  of 65 mm were actually greater than expected (compared to results of SL-ST performed on single bricks bonded for 160 mm, granted that lateral brick's surfaces generally show a slightly higher strength), as if a certain contribute is given by a sort of interlocking of a mortar tooth;
- ❑ end-anchored specimens allowed for stable test progress, avoiding brittle detachments detrimental also for the applied instrumentation; this may be significant also toward a future standardization of bond tests;
- ❑ as expected, stiffer reinforcements (CFRP and SRP) showed higher strength, and equivalent composites (GFRP and BFRP) had similar results.



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